

ERRATA

PCI Design Handbook Precast and Prestressed Concrete Fourth Edition — MNL-120-92

In 1992, the Precast/Prestressed Concrete Institute published the fourth edition of the *PCI Design Handbook — Precast and Prestressed Concrete*. Although very careful efforts were made to provide a "letter-perfect" document, alas, glitches began to show up. What follows is a list of errata, most of which came to light during a round of seminars following publication of the handbook.

As the handbook gets used and tested in the workplace, additional errata items may be spotted. If so, you are urged to send them in, along with any questions or concerns you may have on any material presented in the handbook. Also, your thoughts on any new material you feel should be included in future editions are equally welcome. Please direct your comments to Phillip J. Iverson, PCI's technical director, at PCI headquarters.

1. Page 1-23 — Change Eq. 1.3.2 to read:

$$E_c = w^{1.5} 33 \sqrt{f'_c}$$

2. Page 2-21 — Shift load capacities and cambers for a 10DT24 + 2 with strand pattern 108-D1 one column to the left, that is, 141 psf at 40 ft, 123 psf at 42 ft, etc.
3. Page 3-75 — The missing shearwall dimension on left of plan is 20' - 0".
4. Page 3-78 — Right column, length of weld calculation should be:
- $$\ell_w = 1.33(60)(1.39) / \left[1.67(21)(0.70) \left(\frac{5}{16} \right) \right]$$
- = 14.46 in.; use $\ell_w = 15$ in.
5. Page 4-59 — In Step 3 under Analysis, add ϕ to the right side of the equation for ϕM_n .
6. Page 6-14 — Right column, under bracket plate design, the general equation for t and the example expression for t should show $-2e$ and $-2(2)$ outside the square root sign. The calculation was correctly carried out to give $t = 0.51$ in.
7. Page 6-18 — In the footnote to the figure, the correct correction ratio is $\bar{y}/(d - \bar{y})$.

8. Page 6-28 — In Fig. 6.11.1, the term "Framing bar" should be added to the arrow pointing to the bent vertical bar.
9. Page 6-33 and p. 6-35 — In Figs. 6.13.1 and 6.13.2, the vertical shear steel, A_v and A_{sh} , should be drawn on the outside of the horizontal bars, A_h and A_s . In Fig. 6.13.2, the welded anchor bar should be shown to anchor the A_s bars (4-#5's).
10. Page 6-71 — In Table A, make the following corrections:
- Line 4 - change 5/6 to 5/8; change 11.79 to 12.53
- Line 6 - change 26.09 to 29.07
11. Page 6-71 — In Table B, make the following corrections:
- Line 1 - change 1.77 to 2.33
- Line 3 - change 6.38 to 5.38
- Line 6 - change 10.98 to 10.58; change 13.80 to 13.91
- Line 7 - change 16.44 to 20.12
- Line 9 - change 18.10 to 28.11
12. Page 6-73 — For the channel section, the expression for plastic modulus should be:
- $$bt(h-t) + \frac{w(h-2t)^2}{4}$$
13. Page 6-76 and p. 6-78 — In Table 6.20.16, values for $b = 12$ in. and $\ell_p = 8$ in. are incorrect; in Table 6.20.16, all values for $b = 16$ in. are incorrect. For correct values see PCI MNL-123-88, *Design and Typical Details of Connections for Precast and Prestressed Concrete*, Table A-28, pp. A-20 through A-24.
14. Page 6-79 — The A_s reinforcing bars for $b = 18$ in. and $b = 20$ in. should be tabbed: 2-#6, 2-#7, 2-#8, 2-#9, 3-#6, 3-#7, 3-#8, 3-#9, 4-#6, 4-#7, 4-#8, 4-#9.
15. Page 11-31 — The multiplier for converting kips per linear ft to kg/m is 1488, not 0.001488.